

We have

$$dy = \nabla y(x) dx,$$

where $\nabla y(x)$ is the deformation gradient.

With index notation:

$$dy_i = y_{i,j} dx_j.$$

Therefore

$$dl^2 = |\overline{CD}|^2 = dy_i dy_i$$

$$= y_{i,k} dx_k \cdot y_{i,l} dx_l$$

$$= y_{i,k} y_{i,l} dx_k dx_l.$$

On the other hand

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$$(ds)^2 = (\overline{AB})^2 = dx_j \cdot dx_j = \delta_{ij} dx_i dx_j$$

Hence,

$$(dl)^2 - (ds)^2 = (y_{i,j} y_{i,j} - \delta_{ij}) dx_i dx_j$$

With matrices:

$$(dl)^2 - (ds)^2 = \underline{dx}^T (\nabla_y^T \nabla_y - \underline{I}) \underline{dx}$$

Substituting the expression for the displacement

$$\underline{u}(x) = y(x) - \underline{x} \iff y(x) = \underline{x} + \underline{u}(x)$$

or

$$y_i = x_i + u_i$$

we obtain

$$y_{i,j} = \delta_{i,j} + \delta_{i,j} u_{,k}$$

$$\nabla_y = \nabla_x + \underline{I}$$

and

$$\begin{aligned}
 & y_{i,a} y_{i,e} - \delta_{ae} \\
 &= (u_{i,a} + \delta_{ia}) (u_{i,e} + \delta_{ie}) - \delta_{ae} \\
 &= u_{i,a} u_{i,e} + u_{a,i} + u_{e,i} + \delta_{ia} \delta_{ie} - \delta_{ae} \\
 &= u_{i,a} u_{i,e} + u_{a,i} + u_{e,i}.
 \end{aligned}$$

We denote

$$2 \varepsilon_{ij} = (u_{i,j} + u_{j,i} + u_{k,i} u_{k,j})$$

and it holds

$$\begin{aligned}
 (dl)^2 - (ds)^2 & \\
 &= 2 \varepsilon_{ij} dx_i dx_j.
 \end{aligned}$$

With matrices:

$$\underline{y} = \underline{x} + \underline{u} \Rightarrow$$

$$\nabla_{\underline{y}} = \underline{I} + \nabla_{\underline{u}}$$

and

$$\begin{aligned}
 (dl)^2 - (ds)^2 &= d\underline{x}^T (\underline{I} + \nabla_{\underline{u}}^T) (\underline{I} + \nabla_{\underline{u}}) d\underline{x} \\
 &= d\underline{x}^T (\nabla_{\underline{u}} + \nabla_{\underline{u}}^T + \nabla_{\underline{u}}^T \nabla_{\underline{u}}) d\underline{x},
 \end{aligned}$$

For small displacement
gradients $\nabla \underline{y}^T \nabla \underline{y} \approx \underline{0}$,
and

$$\Sigma_{ij} \approx e_{ij} = \frac{1}{2} (u_{ij} + u_{ji}).$$

In the sequel we will use the
notation Σ_{ij} instead of e_{ij} .

What is the physical interpretation
of Σ_{ij} ?

We have

$$dy_i dy_i = 2 \Sigma_{ee} dy_e dy_e.$$

In general, if we consider
 \underline{dx} , \underline{dx}' that are mapped
to \underline{dy} and \underline{dy}' we
have

$$dy_i dy_i = 2 \Sigma_{ee} dx_e dx_e'$$

$$\text{Let } dx = dx' = (dx_1, 0, 0)$$

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Then

$$|dl|^2 - (ds)^2$$

$$= 2\varepsilon_{RQ} dx_R dx_Q = 2\varepsilon_{11} |dx_1|^2$$

Hence,

$$|dl|^2 = (2\varepsilon_{11} + 1) |dx_1|^2$$

and

$$\frac{|dl| - |dx_1|}{|dx_1|} = (1 + 2\varepsilon_{11})^{1/2} - 1$$

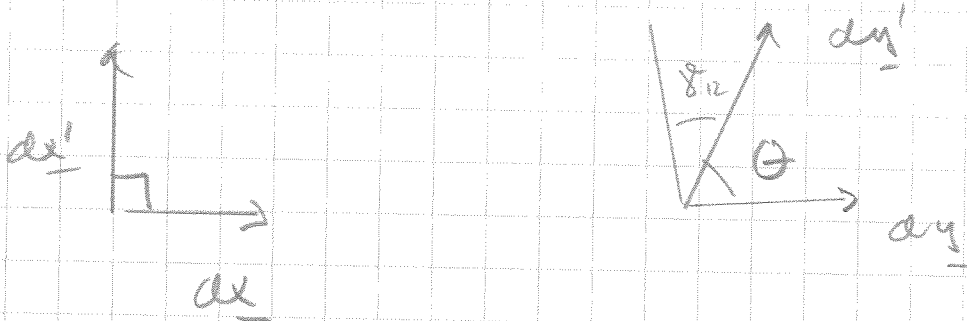
Linearizing, we get

$$\frac{|dl| - |dx_1|}{|dx_1|} \approx \varepsilon_{11}$$

Next, $\varepsilon_{12} \dots$

Let $\underline{dx} = (dx_1, 0, 0)$
 and $\underline{dx}' = (0, dx_2', 0)$
 Then

$$d\underline{y} \cdot d\underline{y}' = 2\varepsilon_{12} dx_1 dx_2'$$



We have

$$\begin{aligned} \cos \theta &= \frac{d\underline{y} \cdot d\underline{y}'}{|d\underline{y}| |d\underline{y}'|} \\ &= \frac{2\varepsilon_{12}}{(1+2\varepsilon_{11})^{1/2} (1+2\varepsilon_{22})^{1/2}} \end{aligned}$$

Let $\gamma_{12} = \pi/2 - \theta$ be the shear angle. It holds

$$\sin \gamma_{12} = \cos \theta = \frac{2\varepsilon_{12}}{(1+2\varepsilon_{11})^{1/2} (1+2\varepsilon_{22})^{1/2}}$$

$$\gamma_{12} \approx \sin \gamma_{12} \approx 2\varepsilon_{12}$$

We have

$$dx_i = u_{ij} dx_j$$

We split

$$u_{ij} = \frac{1}{2} (u_{ij} + u_{ji}) - \frac{1}{2} (u_{ij} - u_{ji})$$

$$= e_{ij} + w_{ij}, \quad \text{with}$$

$$w_{ij} = \frac{1}{2} (u_{ij} - u_{ji}).$$

Hence, the tensor w_{ij} is antisymmetric: $w_{ij} = -w_{ji}$.

Note also by writing

$$\underline{\omega} = (-w_{23}, w_{13}, -w_{12})$$

it holds

$$\underline{\omega} \times \underline{s} = w_{ij} s_j = \underline{\omega} \times \underline{s}.$$

Since

$$\underline{\omega} \times \underline{s} = \begin{pmatrix} w_{13} s_3 + w_{12} s_2, \\ w_{23} s_3 - w_{12} s_1, \\ -w_{23} s_2 - w_{13} s_1 \end{pmatrix}$$

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Hooke's Law

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We assume that the stress depends linearly on the strain.

$$\sigma_{ij} = \sum_{a=1}^3 \sum_{l=1}^3 C_{ij,al} \epsilon_{al}$$

or

$$\sigma_{ij} = C_{ij,kl} \epsilon_{kl}$$

or

$$\underline{\underline{\sigma}} = \underline{\underline{D}} \underline{\underline{\epsilon}} \quad (4)$$

Number of coefficients:

but

$$C_{ij,kl} = C_{jil,k}$$

and

$$C_{ij,kl} = C_{ijlk},$$

From energy considerations one can prove that

$$C_{ij,kl} = C_{klij}$$

This reduces the independent constants to 21,

We define an isotropic material a material for which the coefficients C_{ijkl} are independent of the chosen coordinate system.

It holds

Theorem. For an isotropic material there is two positive constants (Lamé) μ, λ such that

$$\sigma_{ij} = 2\mu \varepsilon_{ij} + \lambda \sum_{kk} \delta_{ij}.$$

Remark. $\mu = G =$ the shear modulus
 $=$ Lame modulus.

Proof. We first prove that for an isotropic material the principal directions for the stress and strain coincide.

We choose the coordinate system according to the principal stresses.

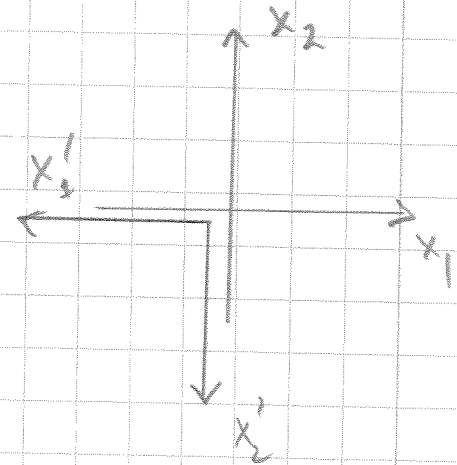
Hence, $\varepsilon_{12} = \varepsilon_{13} = \varepsilon_{23} = 0.$

We need prove that (e.g.)

$$\tau_{23} = 0,$$

We choose a new coordinate system by rotating 180° around the x_3 -axis

The matrix of the coordinate transform is



$$\underline{\underline{A}} = \begin{pmatrix} -1 & 0 & 0 \\ 0 & -1 & 0 \\ 0 & 0 & 1 \end{pmatrix}.$$

We get

$$\underline{\underline{\Sigma}}' = \underline{\underline{A}} \underline{\underline{\Sigma}} \underline{\underline{A}}^T = \begin{pmatrix} \epsilon_{11} & 0 & 0 \\ 0 & \epsilon_{22} & 0 \\ 0 & 0 & \epsilon_{33} \end{pmatrix},$$

i.e. $\epsilon'_{ii} = \epsilon_{ii}$ and

$$\epsilon'_{ij} = 0 \quad i \neq j.$$

For the stress tensor we have

$$\underline{\underline{\sigma'}} = \underline{\underline{A}} \underline{\underline{\sigma}} \underline{\underline{A}}^T = \begin{pmatrix} \sigma_{11} & \sigma_{12} & -\sigma_{13} \\ \sigma_{12} & \sigma_{22} & -\sigma_{23} \\ -\sigma_{13} & -\sigma_{23} & \sigma_{33} \end{pmatrix},$$

Hence, $\sigma'_{23} = -\sigma_{23}$.

From the assumption of isotropy we obtain

$$\begin{aligned} \sigma'_{23} &= A \varepsilon'_{11} + B \varepsilon'_{22} + C \varepsilon'_{33} \\ &= A \varepsilon_{11} + B \varepsilon_{22} + C \varepsilon_{33} \\ &= \sigma_{23}. \end{aligned}$$

We thus have $\sigma'_{23} = -\sigma_{23} \Rightarrow$
 $\sigma_{23} = 0$

In the same way we obtain

$$\sigma'_{13} = 0 \quad \text{and} \quad \sigma'_{12} = 0.$$

Next, we consider (e.g.) the component σ_{11} . We have

$$\sigma_{11} = a \varepsilon_{11} + b \varepsilon_{22} + c \varepsilon_{33}.$$

We rotate the coordinate system

90° , around the x_1 -axis

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so that $x_1' = x_1$, $x_2' = x_3$, $x_3' = -x_2$.

$$\underline{\underline{A}} = \begin{pmatrix} 1 & 0 & 0 \\ 0 & 0 & 1 \\ 0 & -1 & 0 \end{pmatrix}.$$

we obtain

$$\underline{\underline{\sigma'}} = \underline{\underline{A}} \underline{\underline{\sigma}} \underline{\underline{A}} = \begin{pmatrix} \sigma_{11} & 0 & 0 \\ 0 & \sigma_{33} & 0 \\ 0 & 0 & \sigma_{22} \end{pmatrix}$$

Similarly $\epsilon_{11}' = \epsilon_{11}$, $\epsilon_{22}' = \epsilon_{33}$

and $\epsilon_{33}' = \epsilon_{22}$.

Hence, we have

$$\begin{aligned} \sigma_{11}' &= a \epsilon_{11}' + b \epsilon_{22}' + c \epsilon_{33}' \\ &= a \epsilon_{11} + b \epsilon_{33} + c \epsilon_{22}. \end{aligned}$$

On the other hand

$$\sigma_{11}' = \sigma_{11} = a \epsilon_{11} + b \epsilon_{22} + c \epsilon_{33},$$

which implies

$$b = c.$$

We write

$$\begin{aligned} \sigma_{11} &= a \epsilon_{11} + b (\epsilon_{22} + \epsilon_{33}) \\ &= \underbrace{(a-b)}_{:= 2\mu} \epsilon_{11} + \underbrace{b}_{:= \lambda} (\epsilon_{11} + \epsilon_{22} + \epsilon_{33}) \end{aligned}$$

In the same way we conclude that

$$\sigma_{22} = 2\mu \varepsilon_{22} + \lambda \varepsilon_{kk},$$

$$\sigma_{33} = 2\mu \varepsilon_{33} + \lambda \varepsilon_{kk}.$$

In the principal axis system we thus have

$$\underline{\underline{\sigma}} = 2\mu \underline{\underline{\varepsilon}} + \lambda \text{tr}(\underline{\underline{\varepsilon}}) \underline{\underline{I}}$$

and the final result is obtained by transforming to a general coordinate system:

$$\begin{aligned} \underline{\underline{\sigma}}' &= \underline{\underline{A}} \underline{\underline{\sigma}} \underline{\underline{A}}^T = 2\mu \underline{\underline{A}} \underline{\underline{\varepsilon}} \underline{\underline{A}}^T \\ &+ \lambda \text{tr}(\underline{\underline{\varepsilon}}) \underline{\underline{A}} \underline{\underline{A}}^T \\ &= 2\mu \underline{\underline{A}} \underline{\underline{\varepsilon}} \underline{\underline{A}}^T + \lambda \text{tr}(\underline{\underline{A}} \underline{\underline{\varepsilon}} \underline{\underline{A}}^T) \underline{\underline{I}} \\ &= 2\mu \underline{\underline{\varepsilon}}' + \lambda \text{tr}(\underline{\underline{\varepsilon}}') \underline{\underline{I}} \quad \square. \end{aligned}$$

There are quite a few ways of choosing the two material constants, e.g. the following are used:

The bulk modulus:

$$K = k = \lambda + \frac{2}{3}\mu.$$

Young's modulus (Kirchhoff's)

$$E = \frac{(3\lambda + 2\mu)\mu}{\mu + \lambda}$$

The Poisson ratio:

$$\nu = \frac{\lambda}{2(\lambda + \mu)}.$$

In engineering one usually use E and ν . With these it holds

$$\underline{\underline{\underline{\epsilon}}} = \frac{1}{E} \left[(1 + \nu) \underline{\underline{\underline{\sigma}}} - \nu \operatorname{tr}(\underline{\underline{\underline{\sigma}}}) \underline{\underline{\underline{I}}} \right].$$

Conservation of energy.

Uniqueness of the dynamic sol.

The equations of motion for the linear elasticity problem are

$$\rho \ddot{u}_i = (2\mu \varepsilon_{ij}(u) + \lambda \varepsilon_{kk}(u) \delta_{ij})_{,j} + f_i$$

where we have substituted the constitutive law

$$\sigma_{ij} = 2\mu \varepsilon_{ij} + \lambda \varepsilon_{kk} \delta_{ij}$$

into the eq's of motion

$$\rho \ddot{u}_i = \sigma_{ij,j} + f_i$$

Since $\varepsilon_{ij} = \varepsilon_{ji}$ the rotational equations of motion $\sigma_{ij} = \sigma_{ji}$ are satisfied.

Remark. We can develop the right hand side:

$$\begin{aligned}
& (2\mu \varepsilon_{ij}(u) + \lambda \varepsilon_{kk}(u))_{,j} \\
&= (\mu (u_{ij} + u_{ji}) + \lambda u_{kk})_{,j} \\
&= \mu (u_{i,jj} + u_{j,ji}) + \lambda u_{j,ji} \\
&= \mu u_{i,jj} + (\mu + \lambda) u_{j,ji}
\end{aligned}$$

For the time-independent problem
we get the Navier/Camé equations

$$\mu u_{i,jj} + (\mu + \lambda) u_{jji} + f_i = 0.$$

Note that $u_{i,jj} = \Delta u_i$

and $u_{jji} = \operatorname{div} \underline{u}$.

Hence, the equations are

$$\mu \Delta \underline{u} + (\mu + \lambda) \nabla (\operatorname{div} \underline{u}) + \underline{f} = \underline{0}.$$

It is preferable not to use these forms as they do not "conform the mechanical structure" of the problem. \square

To the equations of motion

we still have to add:

The initial conditions at the

time $t = 0$:

$$\underline{u}(x, 0) = \underline{u}^0(x) \quad x \in \Omega,$$

$$\dot{\underline{u}}(x, 0) = \underline{v}^0(x) \quad x \in \Omega,$$

and

the boundary conditions, e.g.,
for $\partial\Omega = \Gamma_D \cup \Gamma_N$ (Dirichlet,
Neumann):

$$\underline{u}(\underline{x}, t) = \underline{g}(\underline{x}, t) \quad \underline{x} \in \Gamma_D.$$

$$\underline{\underline{\sigma}} \underline{n} = \underline{t}(\underline{x}, t) \quad \underline{x} \in \Gamma_N,$$

The 2nd condition is hence

$$(2\mu \varepsilon_{ij}(\underline{u}) + \lambda \varepsilon_{kk}(\underline{u}) \delta_{ij}) n_j = t_i.$$

We arrived at the problem:

Find $\underline{u}(\underline{x}, t) = u_i(\underline{x}, t)$ such that

$$\rho \ddot{u}_i = (2\mu \varepsilon_{ij}(\underline{u}) + \lambda \varepsilon_{kk}(\underline{u}))_{,j} + f_i$$

in Ω ,

$$\left. \begin{aligned} u_i(\underline{x}, 0) &= u_i^0(\underline{x}) \\ \dot{u}_i(\underline{x}, 0) &= v_i^0(\underline{x}) \end{aligned} \right\} \underline{x} \in \Omega.$$

$$u_i(\underline{x}, t) = g_i(\underline{x}, t), \quad \underline{x} \in \Gamma_D,$$

$$2\mu \varepsilon_{ij}(\underline{u}) n_j + \lambda \varepsilon_{kk}(\underline{u}) n_j = t_i \quad \underline{x} \in \Gamma_N.$$

Let us define the elastic energy per unit volume:

$$U_0 = \frac{1}{2} c_{ijkl} \varepsilon_{ij}(\underline{u}) \varepsilon_{kl}(\underline{u})$$

and the elastic energy of the body

$$\begin{aligned} U(\underline{u}) &= \frac{1}{2} \int_{\Omega} c_{ijkl} \varepsilon_{ij} \varepsilon_{kl} \, dx \\ &= \int_{\Omega} U_0 \, dx, \end{aligned}$$

The total kinetic energy is

$$\begin{aligned} K(\underline{u}) &= \frac{1}{2} \int_{\Omega} \rho \dot{u}_i \dot{u}_i \, dx \\ &= \frac{1}{2} \int_{\Omega} \rho |\dot{\underline{u}}|^2 \, dx. \end{aligned}$$

The potential energy of the load is

$$\int_{\Omega} f_i u_i \, dx \quad (= \int_{\Omega} \underline{f} \cdot \underline{u} \, dx).$$

We derive the following

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Theorem. Suppose that the loads f_i and t_i , and the displacement g_i varies. Then the total energy is constant in time

$$U(y)(t) + K(y)(t) = C \neq C(t).$$

Proof. Let us compute:

$$\frac{d}{dt} [U + K] = ?$$

$$\frac{d}{dt} K(y)(t) = \frac{d}{dt} \frac{1}{2} \int_{\Omega} g_i \dot{u}_i \dot{u}_i dx$$

$$= \frac{1}{2} \int_{\Omega} g_i (\ddot{u}_i \dot{u}_i + \dot{u}_i \ddot{u}_i) dx$$

$$= \int_{\Omega} g_i \ddot{u}_i \dot{u}_i dx$$

$$\frac{d}{dt} U(y) = \frac{1}{2} \frac{d}{dt} \int_{\Omega} c_{ijkl} \varepsilon_{ij}(y) \varepsilon_{kl}(y) dx$$

$$= \frac{1}{2} \int_{\Omega} [c_{ijkl} \varepsilon_{ij}(\dot{y}) \varepsilon_{kl}(y) +$$

$$C_{ijkl} \sum_{ij} (\underline{u}) \sum_{kl} (\underline{u}) \int dx$$

$$= \int_{\Omega} C_{ijkl} \sum_{kl} (\underline{u}) \sum_{ij} (\underline{u}) dx$$

(since , we have $C_{ijkl} = C_{klij}$!)

$$= \int_{\Omega} \sigma_{ij} \sum_{ij} (\underline{u}) dx$$

$$= \frac{1}{2} \int_{\Omega} \sigma_{ij} (u_{ij} + u_{ji}) dx$$

(use the exercise)

$$= \int_{\Omega} \sigma_{ij} u_{ij} dx .$$

Next, we "integrate by parts".

$$\int_{\Omega} \sigma_{ij} u_{ij} dx$$

$$= - \int_{\Omega} \sigma_{ij,j} u_i dx + \int_{\partial \Omega} \sigma_{ij} n_j u_i ds$$

$$-\int_{\Omega} \sigma_{ij,j} \bar{u}_i \, dx + \int_{\Gamma_N} \underbrace{\sigma_{ij} n_j}_{=t_i = 0} \bar{u}_i \, dS$$

$$+ \int_{\Gamma_D} \sigma_{ij} n_j \bar{u}_i \, dS$$

" $\ddot{g}_i = 0$

$$= -\int_{\Omega} \sigma_{ij,j} \bar{u}_i \, dx$$

We thus have

$$\frac{d}{dt} [K(\underline{u}) + U(\underline{u})]$$

$$= \int_{\Omega} [\rho \ddot{u}_i - \sigma_{ij,j}] \bar{u}_i \, dx$$

$$= \int_{\Omega} f_i \bar{u}_i \, dx = 0$$

□

In general we assume that the elastic energy is non-negative

$$\int_{\Omega} C_{ijkl} \varepsilon_{ij}(\underline{u}) \varepsilon_{kl}(\underline{u}) \, dx \geq 0$$

Or stronger, the ellipticity condition $\exists \alpha > 0$ s.e.

$$C_{ijkl} S_{ij} S_{kl} \geq \alpha S_{ij} S_{ij} \\ = \alpha |S|^2 \quad \forall S_{ij}$$

Since $\mu > 0$, $\lambda \geq 0$ this is true for isotropic materials.

Corollary. The solution to the dynamic problem is unique.

Proof. By linearity we have to show that $\underline{u}^0 = \underline{0}$, $\underline{u}^0 = \underline{0}$, $\underline{t} = \underline{0}$, $\underline{q} = \underline{0}$ and $\underline{t} = \underline{0}$ implies that

$$\underline{u} \equiv \underline{0}$$

We have

$$U(\underline{u})(0) + K(\underline{u})(0) = 0.$$

and hence

$$U(\underline{u})(t) + K(\underline{u})(t) = 0.$$

That is

$$\frac{1}{2} \int_{\Omega} c_{ijkl} \varepsilon_{ij}(u) \varepsilon_{kl}(u) dx$$

$$+ \frac{1}{2} \int_{\Omega} \rho \dot{u}_i \dot{u}_i dx = 0$$

Both integrals are non-negative.

(Due to the ellipticity it holds

$$\int_{\Omega} c_{ijkl} \varepsilon_{ij}(u) \varepsilon_{kl}(u) dx$$

$$\geq \alpha \int_{\Omega} \varepsilon_{ij}(u) \varepsilon_{ij}(u) dx \geq 0.)$$

Hence \underline{u} is a rigid body dis-

placement

$$\underline{u}(x, t) = \underline{a}(t) + \underline{b}(t) \times \underline{x},$$

but independent of time

$$\underline{\dot{u}} = \underline{0}.$$

Hence

$$\underline{u}(x, t) = \underline{a} + \underline{b} \times \underline{x}.$$

The initial condition then implies $\underline{u} \equiv \underline{0}$.